

# **Informing Water Treatment Plant Design with Localized Hydraulic Models**

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#### Agenda

- Importance of Hydraulics in Treatment Plant Design
  - General Overview
  - Flow Distribution
- Practical Examples of Design Decisions Informed by Hydraulics
- Q&A

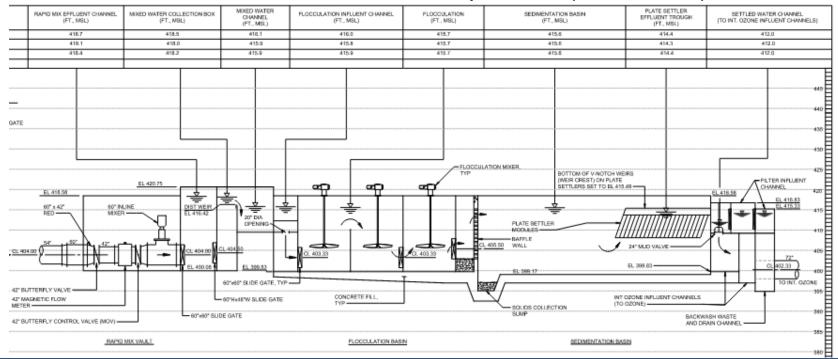


Importance of Hydraulics in Treatment Plant Design

### What is a Hydraulic Profile?

Determine headloss as water flows through each process

#### Establish Hydraulic Grade Lines (HGLs) through the treatment process (water level)



#### **Reasons for Preparing Hydraulic Profiles**

- To ensure the hydraulic gradient is adequate for flow through the treatment facility
- To prevent facility overflows during peak flow events
- To establish head requirements for pump design
- To ensure equal flow distribution between processes





## **Design Flows**

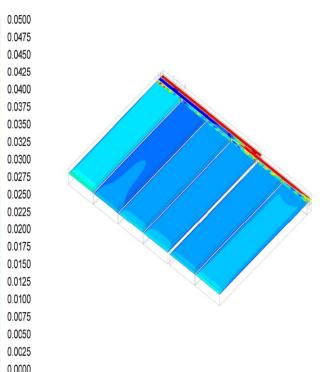
- Minimum Flow
  - Consider minimum velocities to be maintained
  - Consider for control valve and pump operation
- Average Flow
  - Sets typical depths and volumes
- Rated Plant Capacity
  - Used to design wall elevations and contain water levels within tanks and channels
  - Consider "worst case" condition
    - Process unit out of service?
    - Peak instantaneous flow?

#### COMPLETE HYDRAULIC CALCULATIONS FOR ALL PERTINENT FLOW CONDITIONS



## **Flow Distribution and Control**

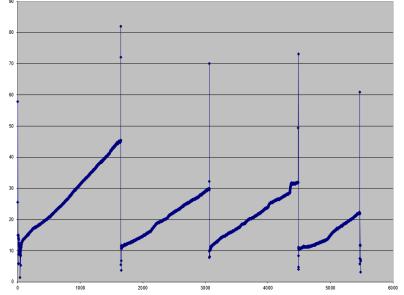
- Benefits of uniform flow distribution:
  - Best use of available hydraulic capacity
  - Improves process performance and reliability
  - Contributes to consistent compliance with tighter effluent limits
- Symmetry, although helpful, does not ensure proper flow distribution
- Maintaining flow distribution
  - Inducing headloss at points of desired distribution (i.e. orifices/ports, weirs, rate-of-flow controller)



#### **Additional Hydraulic Considerations**

- Provide adequate detention/contact times and loading rates in process units
- Consider plans for future expansion (ex. sizing of pipes and channels for future flows)
- Variable headloss conditions (clean/dirty filters, screens, etc.)

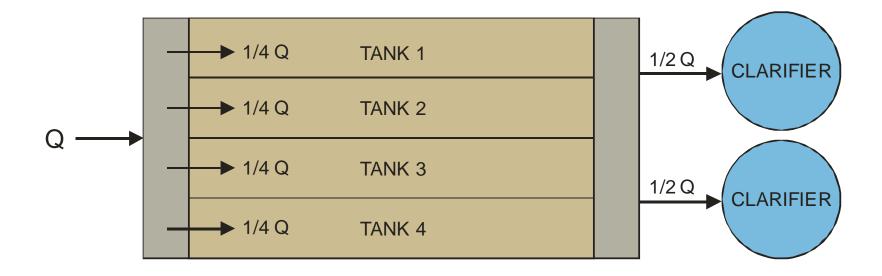
## Filter HL vs. Time



Hydraulic Overview: Flow Distribution

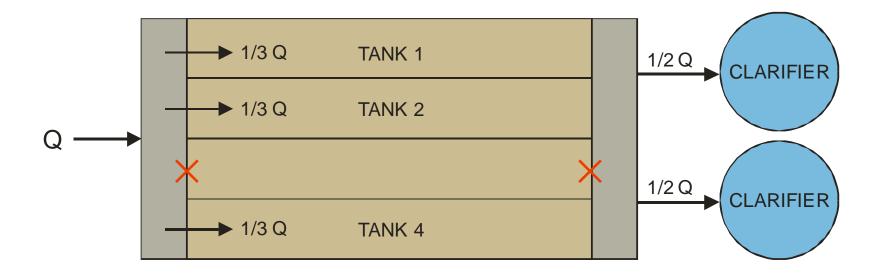
#### **Flow Distribution - Objectives**

• Flow Distribution – 95% distribution to all units



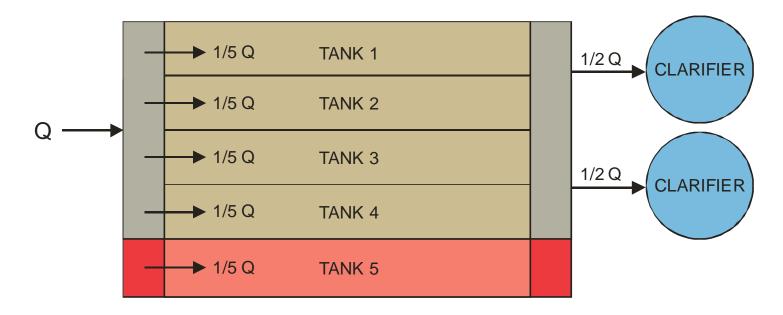
#### **Flow Distribution - Objectives**

- Flow Distribution 95% distribution to all units
  - Ease of operation



#### **Flow Distribution - Objectives**

- Flow Distribution 95% distribution to all units
  - Expandability



## **Maintaining Flow Distribution**

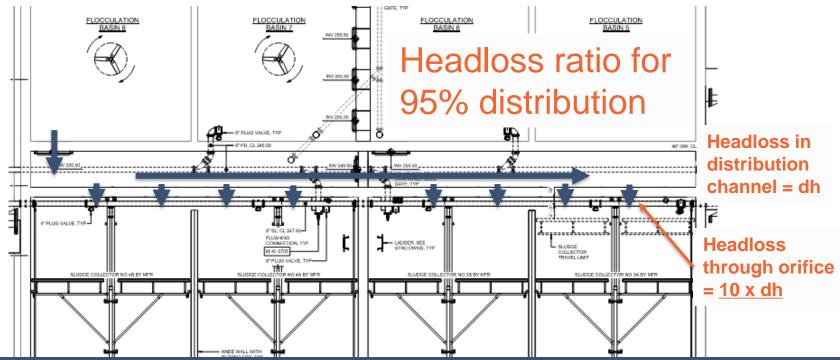
• Basic Principle – Induce Headloss!

Provide a high enough headloss at points where equal flow distribution is required so that the head differential acting against equal flow distribution is relatively small in comparison.

- Common ways to induce headloss
  - Weirs (splitter boxes, etc.)
  - Ports (distribution from channel to process units)
  - Rate Controllers (valves, etc.)
    - Not always desirable in processes other than filters
    - Additional equipment cost and maintenance required

## **Flow Distribution**

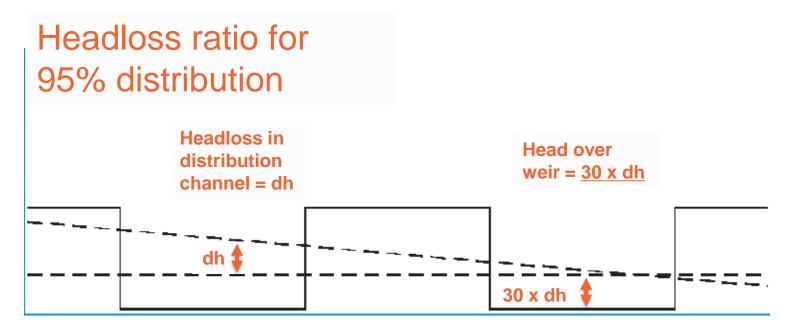
- Orifices
  - Headloss in channel between orifices (dh)
  - Headloss through orifice: 10 x dh



## **Flow Distribution**

• Weirs

- Headloss in channel between weirs (dh)
- Head over weir: 30 x dh



# Practical Examples

Example – Williams WTP

Identifying hydraulic capacity of aging WTP





Example – Williams WTP

Identifying hydraulic capacity of aging WTP

Developed hydraulic model of existing plant with record drawings

• Model calibrated with field measured water elevations

| Process                                     | Measured WSE (ft) | Hydraulic Profile WSE (ft) | Model<br>Error (ft) |
|---|-------------------|----------------------------|---------------------|
| Flocculation Basins (West Train)            | 389.80            | 389.79                     | 0.01                |
| Sedimentation Basin (West)                  | 389.68            | 389.72                     | 0.04                |
| Sedimentation Basin (East)                  | 389.72            | 389.72                     | 0.00                |
| Sedimentation Basin Effluent Channel (West) | 389.33            | 389.30                     | 0.03                |
| Sedimentation Basin Effluent Channel (East) | 389.27            | 389.27                     | 0.00                |
| Filter Influent Channel (West)              | 389.27            | 389.26                     | 0.01                |
| Filter Influent Channel (East)              | 389.20            | 389.26                     | 0.06                |
| Filters                                     | 388.63            | 388.63                     | 0.00                |

Results of Existing WTP Hydraulic Model Calibration at 6.5 mgd Flow

Example – Williams WTP

Defined freeboard at various flow rates up to permitted capacity of 22 MGD. Red cells mean < 12" freeboard

| Flow<br>(mgd) | Filtered Water<br>Collection Box | Filters | Filter<br>Influent<br>Weirs | Sed Basin<br>Effluent<br>Weirs | Sed<br>Basins    | Sed Basin<br>Influent<br>Channel | Flocculation<br>Basins | Rapid<br>Mix |
|---------------|----------------------------------|---------|-----------------------------|--------------------------------|------------------|----------------------------------|------------------------|--------------|
| 8             | 3.8                              | 2.2     | 0.4                         | 0.2 <sup>2</sup>               | 1.1              | 1.0                              | 2.1                    | 2.1          |
| 10            | 3.5                              | 2.2     | 0.4                         | 0.1 <sup>2</sup>               | 1.1              | 0.9 <sup>2</sup>                 | 2.1                    | 2.1          |
| 12            | 3.2                              | 2.2     | 0.3 <sup>2</sup>            | 0.0 <sup>2</sup>               | 1.0              | 0.8 <sup>2</sup>                 | 2.0                    | 2.0          |
| 14            | 2.8                              | 2.2     | 0.2 <sup>2</sup>            | -0.1 <sup>2</sup>              | 1.0              | 0.7 <sup>2</sup>                 | 1.9                    | 1.9          |
| 22            | 0.8 <sup>2</sup>                 | 2.2     | 0.0 <sup>2</sup>            | -0.3 <sup>2</sup>              | 0.9 <sup>2</sup> | 0.4 <sup>2</sup>                 | 1.4                    | 1.4          |

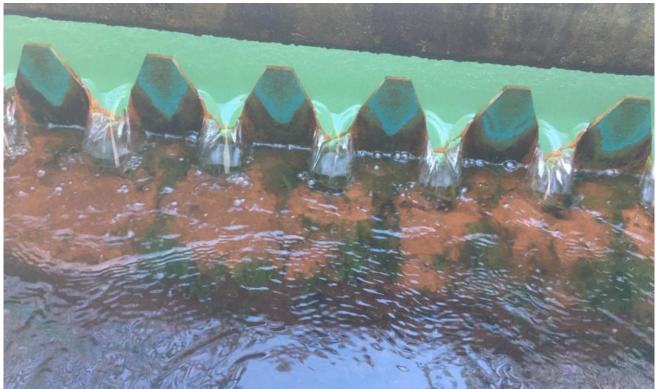
Existing WTP Model Results – Freeboard<sup>1</sup> Throughout WTP Processes (ft)

<sup>1</sup>Freeboard is defined as the distance between water level and top of concrete or downstream water level and weir elevation.

<sup>2</sup>Red shaded cells indicate less than 12 inches of freeboard from top of concrete elevation or less than 4 inches of downstream freeboard from the weir elevation.

Example – Williams WTP

#### Limited freeboard!

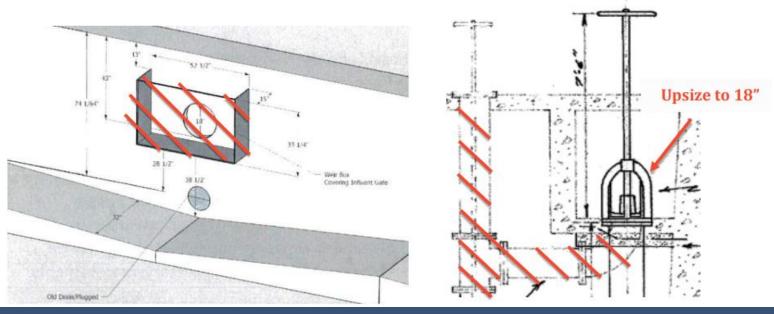




Example – Williams WTP

Recommended improvements to remedy hydraulic bottlenecks

- Upsize sedimentation basin influent mud valves
- Remove unnecessary filter influent weirs



Example – Williams WTP

Resulting freeboard predicted by model with recommended improvements

WTP with Recommended Improvements Model Results – Freeboard<sup>1</sup> Throughout WTP Processes (ft)

| Flow<br>(mgd) | Filtered Water<br>Collection Box | Filters | Filter<br>Influent<br>Weirs | Sed Basin<br>Effluent<br>Weirs | Sed<br>Basins | Sed Basin<br>Influent<br>Channel | Flocculation<br>Basins | Rapid<br>Mix |
|---------------|----------------------------------|---------|-----------------------------|--------------------------------|---------------|----------------------------------|------------------------|--------------|
| 8             | 3.8                              | 1.7     | N/A                         | 0.3                            | 1.1           | 1.0                              | 2.2                    | 2.2          |
| 10            | 3.5                              | 1.9     | N/A                         | 0.3                            | 1.1           | 1.0                              | 2.2                    | 2.2          |
| 12            | 3.2                              | 2.0     | N/A                         | 0.3                            | 1.0           | 1.0                              | 2.2                    | 2.1          |
| 14            | 2.8                              | 2.2     | N/A                         | 0.3                            | 1.0           | 1.0                              | 2.1                    | 2.1          |
| 22            | 0.8 <sup>2</sup>                 | 3.0     | N/A                         | 0.3                            | 1.0           | 0.9 <sup>2</sup>                 | 1.9                    | 1.9          |

<sup>1</sup> Freeboard is defined as the distance between water level and top of concrete or downstream water level downstream and weir elevation.

<sup>2</sup> Red shaded cells indicate less than 12 inches of freeboard from top of concrete elevation or less than 4 inches of freeboard from weir elevation.

Example – Cooleemee WTP

Replacing aging WTP with greenfield 3.5 MGD WTP

New WTP is up a hill from existing WTP

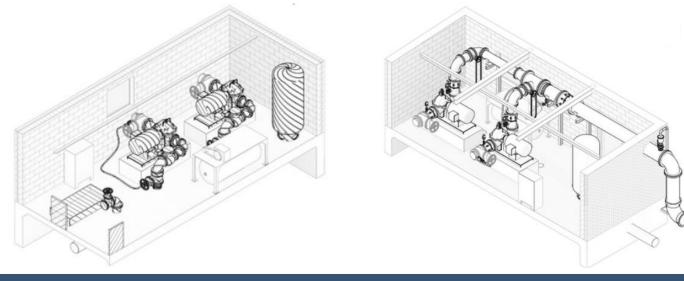
 Need new raw water (RW) pumps with more TDH to get water from river to plant



Example – Cooleemee WTP

KYPIPE model to develop system curve for new RW pumping system

- Increased static head
- New, larger RW transmission pipeline

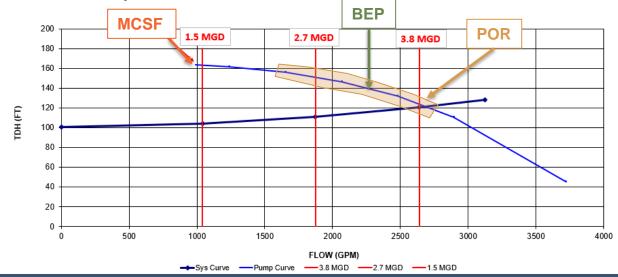




Example – Cooleemee WTP

Select new pumps to meet design points

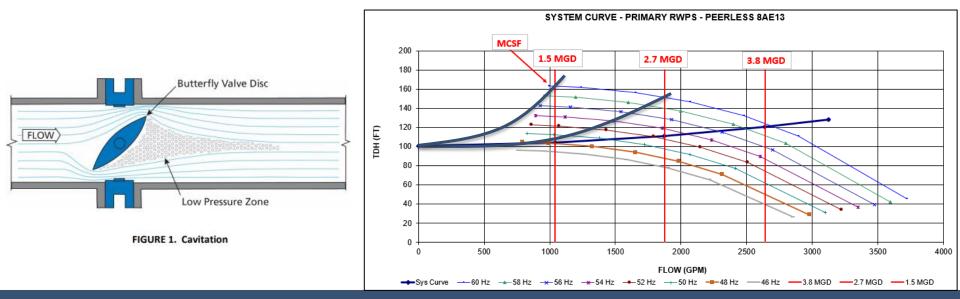
- Minimum flow point keep above min. continuous stable flow
- Average flow point want very good efficiency here
- Maximum flow point be able to meet this w/ reasonable efficiency



Example – Cooleemee WTP

How to control flow rate to meet our 1.5 - 3.8 MGD range?

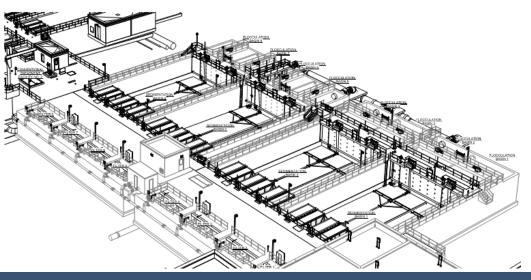
- Control valve *changes system curve*
- Variable Frequency Drives (VFDs) on pumps *changes pump curve*

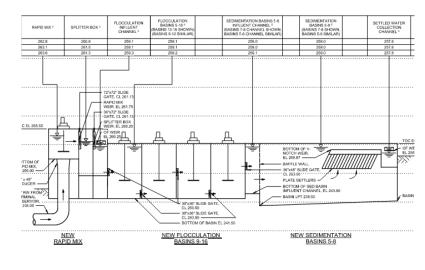


Example – Sanford WTF Expansion

12 MGD 30 MGD (with future 42 MGD in mind)

- 1. Excel-based hydraulic model of existing plant
- 2. Calibrate model with field measurements
- 3. Expand model to include proposed facilities and higher flow rates





Example – Sanford WTF Expansion

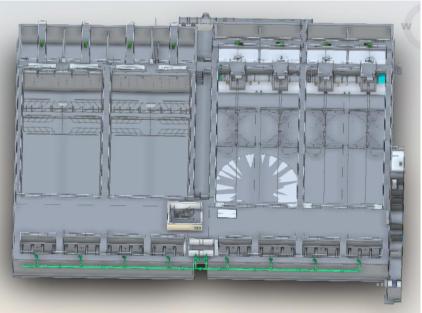
30 MGD (with future 42 MGD in mind)

Sizing for wide range of flows!

• Pipes

12 MGD

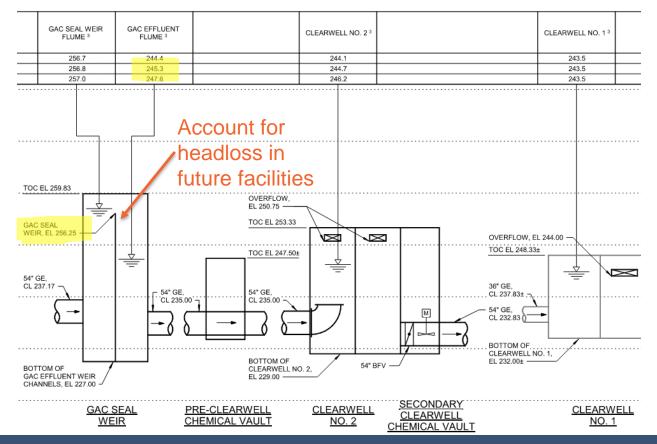
- Channels
- Basins
- Pumps



Ensure solids don't settle out at low flows

Avoid floc shear and excessive headloss at high flows

#### Example – Sanford WTF Expansion



Example – Sanford WTF Expansion

Flow control valves operate best between 20%-80% open.

Size valves to keep operations in that range for worst-case scenarios

Maximum headloss to induce at filters (closed valve)

• Min flow, clean bed, all in service

Minimum headloss to induce at filters (open valve)

• Max flow, dirty bed, 2 out of service

