



# **Informing Water Treatment Plant Design with Localized Hydraulic Models**

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#### **Agenda**

- Importance of Hydraulics in Treatment Plant Design
	- General Overview
	- Flow Distribution
- Practical Examples of Design Decisions Informed by **Hydraulics**
- Q&A





Importance of Hydraulics in Treatment Plant Design

#### **What is a Hydraulic Profile?**

#### Determine headloss as water flows through each process

#### Establish Hydraulic Grade Lines (HGLs) through the treatment process (water level)



#### **Reasons for Preparing Hydraulic Profiles**

- To ensure the hydraulic gradient is adequate for flow through the treatment facility
- To prevent facility overflows during peak flow events
- To establish head requirements for pump design
- To ensure equal flow distribution between processes





## **Design Flows**

- Minimum Flow
	- Consider minimum velocities to be maintained
	- Consider for control valve and pump operation
- Average Flow
	- Sets typical depths and volumes
- Rated Plant Capacity
	- Used to design wall elevations and contain water levels within tanks and channels
	- Consider "worst case" condition
		- Process unit out of service?
		- Peak instantaneous flow?

#### **COMPLETE HYDRAULIC CALCULATIONS FOR ALL PERTINENT FLOW CONDITIONS**



## **Flow Distribution and Control**

- Benefits of uniform flow distribution:
	- Best use of available hydraulic capacity
	- Improves process performance and reliability
	- Contributes to consistent compliance with tighter effluent limits
- Symmetry, although helpful, does not ensure proper flow distribution
- Maintaining flow distribution
	- Inducing headloss at points of desired distribution (i.e. orifices/ports, weirs, rate-of-flow controller)



#### **Additional Hydraulic Considerations**

## Filter HL vs. Time



Hydraulic Overview: Flow Distribution

#### **Flow Distribution - Objectives**

• Flow Distribution – 95% distribution to all units





#### **Flow Distribution - Objectives**

- Flow Distribution 95% distribution to all units
	- Ease of operation





#### **Flow Distribution - Objectives**

- Flow Distribution 95% distribution to all units
	- Expandability





#### **Maintaining Flow Distribution**

• Basic Principle – Induce Headloss!

*Provide a high enough headloss at points where equal flow distribution is required so that the head differential acting against equal flow distribution is relatively small in comparison.*

- Common ways to induce headloss
	- Weirs (splitter boxes, etc.)
	- Ports (distribution from channel to process units)
	- Rate Controllers (valves, etc.)
		- Not always desirable in processes other than filters
		- Additional equipment cost and maintenance required



## **Flow Distribution**

- Orifices
	- Headloss in channel between orifices (dh)
	- Headloss through orifice: 10 x dh



## **Flow Distribution**

• Weirs

- Headloss in channel between weirs (dh)
- Head over weir: 30 x dh



# Practical Examples

Example – Williams WTP *Identifying hydraulic capacity of aging WTP*





Example – Williams WTP

*Identifying hydraulic capacity of aging WTP*

Developed hydraulic model of existing plant with record drawings

• Model calibrated with field measured water elevations

<b>Process</b>	<b>Measured WSE (ft)</b>	<b>Hydraulic Profile WSE (ft)</b>	<b>Model</b> Error (ft)	
<b>Flocculation Basins (West Train)</b>	389.80	389.79	0.01	
<b>Sedimentation Basin (West)</b>	389.68	389.72	0.04	
Sedimentation Basin (East)	389.72	389.72	0.00	
Sedimentation Basin Effluent Channel (West)	389.33	389.30	0.03	
Sedimentation Basin Effluent Channel (East)	389.27	389.27	0.00	
<b>Filter Influent Channel (West)</b>	389.27	389.26	0.01	
<b>Filter Influent Channel (East)</b>	389.20	389.26	0.06	
<b>Filters</b>	388.63	388.63	0.00	

Results of Existing WTP Hydraulic Model Calibration at 6.5 mgd Flow

Example – Williams WTP

Defined freeboard at various flow rates up to permitted capacity of 22 MGD. Red cells mean < 12" freeboard

<b>Flow</b> (mgd)	<b>Filtered Water</b> <b>Collection Box</b>	<b>Filters</b>	<b>Filter</b> <b>Influent</b> <b>Weirs</b>	<b>Sed Basin</b> <b>Effluent</b> <b>Weirs</b>	<b>Sed</b> <b>Basins</b>	<b>Sed Basin</b> <b>Influent</b> <b>Channel</b>	<b>Flocculation</b> <b>Basins</b>	<b>Rapid</b> <b>Mix</b>
8	3.8	2.2	0.4	0.2 <sup>2</sup>	1.1	1.0	2.1	2.1
10	3.5	2.2	0.4	0.1 <sup>2</sup>	1.1	0.9 <sup>2</sup>	2.1	2.1
12	3.2	$2.2\,$	0.3 <sup>2</sup>	0.0 <sup>2</sup>	1.0	0.8 <sup>2</sup>	2.0	2.0
14	2.8	2.2	0.2 <sup>2</sup>	$-0.12$	1.0	0.7 <sup>2</sup>	1.9	1.9
22	0.8 <sup>2</sup>	2.2	0.0 <sup>2</sup>	$-0.32$	0.9 <sup>2</sup>	0.4 <sup>2</sup>	1.4	1.4

Existing WTP Model Results - Freeboard<sup>1</sup> Throughout WTP Processes (ft)

<sup>1</sup> Freeboard is defined as the distance between water level and top of concrete or downstream water level and weir elevation.

<sup>2</sup> Red shaded cells indicate less than 12 inches of freeboard from top of concrete elevation or less than 4 inches of downstream freeboard from the weir elevation.

Example – Williams WTP

#### Limited freeboard!





Example – Williams WTP

Recommended improvements to remedy hydraulic bottlenecks

- Upsize sedimentation basin influent mud valves
- Remove unnecessary filter influent weirs



Example – Williams WTP

Resulting freeboard predicted by model with recommended improvements

> WTP with Recommended Improvements Model Results - Freeboard<sup>1</sup> Throughout WTP Processes (ft)



<sup>1</sup> Freeboard is defined as the distance between water level and top of concrete or downstream water level downstream and weir elevation.

<sup>2</sup> Red shaded cells indicate less than 12 inches of freeboard from top of concrete elevation or less than 4 inches of freeboard from weir elevation.

Example – Cooleemee WTP

*Replacing aging WTP with greenfield 3.5 MGD WTP*

New WTP is up a hill from existing WTP

• Need new raw water (RW) pumps with more TDH to get water from river to plant



Example – Cooleemee WTP

KYPIPE model to develop system curve for new RW pumping system

- Increased static head
- New, larger RW transmission pipeline





Example – Cooleemee WTP

**Hazen** 

Select new pumps to meet design points

- Minimum flow point keep above min. continuous stable flow
- Average flow point want very good efficiency here
- Maximum flow point be able to meet this w/ reasonable efficiency



Example – Cooleemee WTP

How to control flow rate to meet our 1.5 - 3.8 MGD range?

- Control valve *changes system curve*
- Variable Frequency Drives (VFDs) on pumps *changes pump curve*



Example – Sanford WTF Expansion

*12 MGD 30 MGD (with future 42 MGD in mind)*

- 1. Excel-based hydraulic model of existing plant
- 2. Calibrate model with field measurements
- 3. Expand model to include proposed facilities and higher flow rates





Example – Sanford WTF Expansion

*12 MGD 30 MGD (with future 42 MGD in mind)*

#### Sizing for wide range of flows!

- Pipes
- **Channels**
- **Basins**
- Pumps



Ensure solids don't settle out at low flows

Avoid floc shear and excessive headloss at high flows

#### Example – Sanford WTF Expansion



Example – Sanford WTF Expansion

Flow control valves operate best between 20%-80% open.

Size valves to keep operations in that range for worst-case scenarios

Maximum headloss to induce at filters (closed valve)

Min flow, clean bed, all in service

Minimum headloss to induce at filters (open valve)

Max flow, dirty bed, 2 out of service

